

# **PUBLIC NOTICE DETAILS**

### PLANNING APPLICATION DETAILS

Application Number:	DA 2025/29
Application Type:	Discretionary Development Application
Property Location:	45 Cider Gum Road, Miena
Proposal:	Outbuilding
Advertising Commencement Date:	5 September 2025
Representation Period Closing Date:	19 September 2025
Responsible Officer:	Louisa Brown, Senior Planning Officer

The relevant documents may be viewed at Council's website <a href="www.centralhighlands.tas.gov.au">www.centralhighlands.tas.gov.au</a> or at Council's Offices 19 Alexander Street, Bothwell & 6 Tarleton Street, Hamilton during normal business hours.

Enquiries regarding this Application can be made by contacting Central Highlands Council on (03) 6259 5503 or by emailing <a href="mailto:development@centralhighlands.tas.gov.au">development@centralhighlands.tas.gov.au</a>. Please quote the "Application Number" when making your enquiry.

Representations on this application may be made to the General Manager in writing either by:

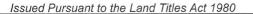
Post: 19 Alexander Street, Bothwell TAS 7030 Email: development@centralhighlands.tas.gov.au

All representations must include the authors full name, contact number and postal address and be received by 5.00pm on the representation period closing date.



## RESULT OF SEARCH

RECORDER OF TITLES





### SEARCH OF TORRENS TITLE

VOLUME	FOLIO
50870	567
EDITION	DATE OF ISSUE
5	27-Mar-2023

SEARCH DATE : 06-Dec-2024 SEARCH TIME : 10.34 AM

## DESCRIPTION OF LAND

Parish of FENWICK, Land District of CUMBERLAND Lot 567 on Sealed Plan 50870 Derivation: Part of Lot 3156 Granted to F. & W. Synott & Part of Lot 29657 Granted to A.J. Drysdale Prior CT 4814/89

### SCHEDULE 1

M758853 TRANSFER to PAUL JOHN CHAMBERS and JESSICA MAREE STECKO Registered 07-Jun-2019 at 12.01 PM

## SCHEDULE 2

Reservations and conditions in the Crown Grant if any SP 50870 COVENANTS in Schedule of Easements SP 50870 FENCING COVENANT in Schedule of Easements E342353 MORTGAGE to AMP Bank Limited Registered 27-Mar-2023 at 12.01 PM

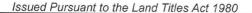
### UNREGISTERED DEALINGS AND NOTATIONS

No unregistered dealings or other notations

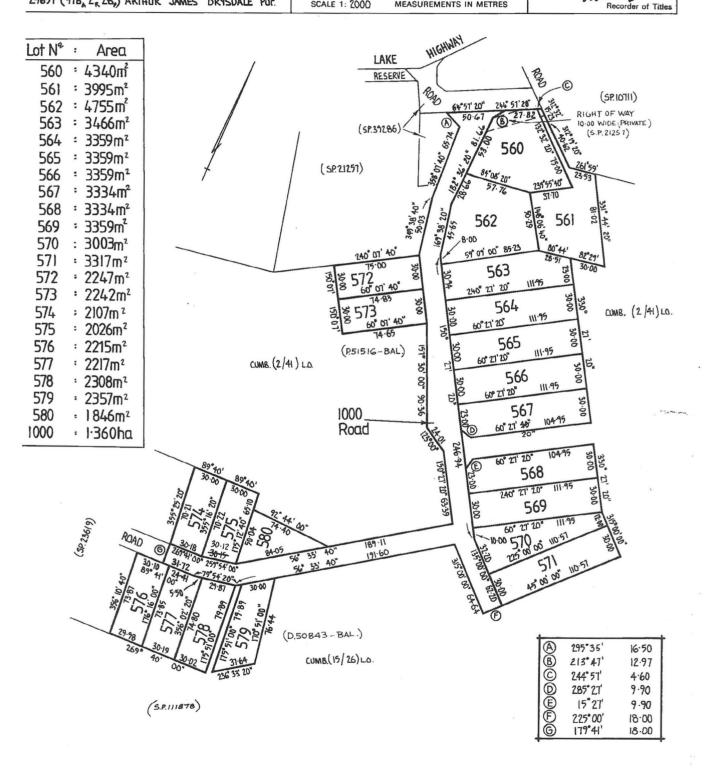


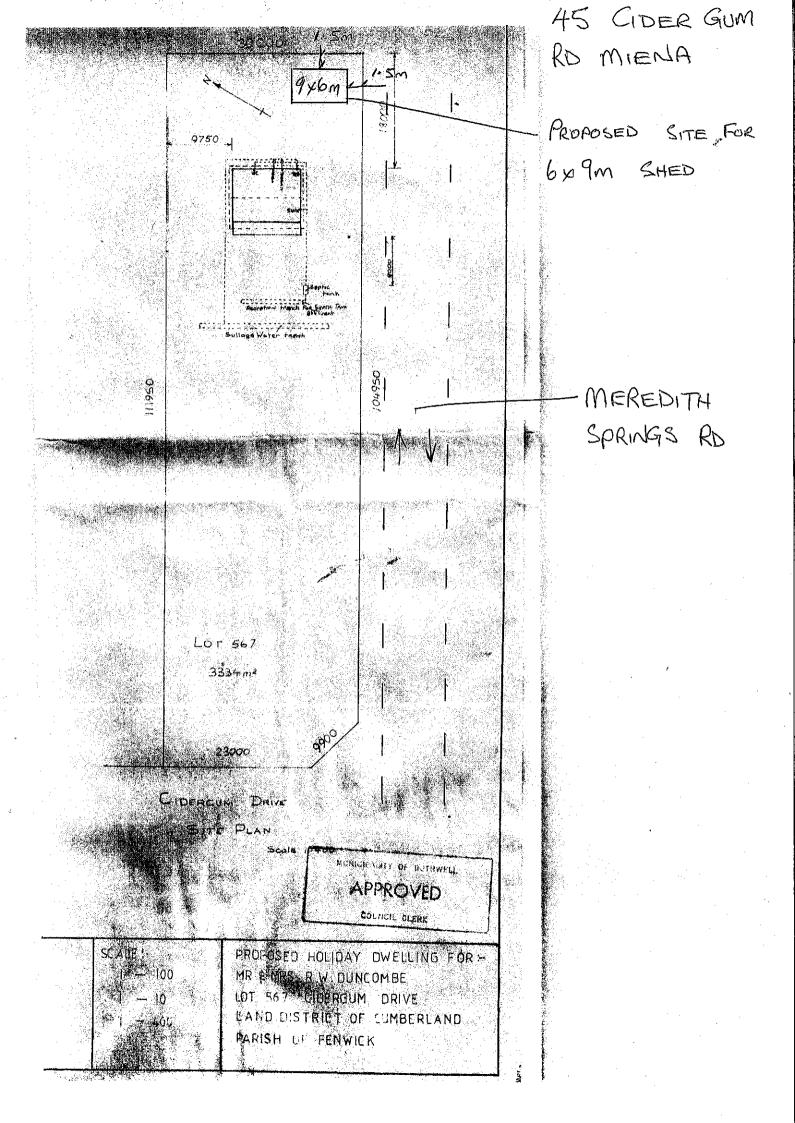
## **FOLIO PLAN**

RECORDER OF TITLES









## STRUCTURAL GENERAL NOTES

#### 1.0 General

- 1.1 These drawings are
  - Jointly owned by HiTen Buildings and Venn Engineering Pty Ltd
  - Provided for the sole purpose of obtaining building approval and guiding construction of a single building at the job address shown in the title block
  - Prohibited to be used for any other purpose without written authorisation from HiTen Buildings and Venn Engineering Pty Ltd.
  - Only valid if signed by the engineer and must not be altered in any way without signed approval from the engineer.
  - Produced to scale but dimensions shall not be obtained by measuring the drawings. All dimensions are in millimeters unless stated otherwise.
- 1.2 The engineer accepts no liability or responsibility for the contents of drawings that are invalid.
- 1.3 The word 'the engineer' used in these notes refers to an employee or nominated representative of Venn Engineering Pty Ltd.
- 1.4 The engineer is not the project manager or site supervisor for this project. It is the responsibility of the project manager or site supervisor for this project. It is the responsibility of the project manager or site supervisor in charge to ensure that the non-structural requirements of the Governing Building Code are considered and appropriately designed. This includes, but not limited to, fire & bushfire design, access requirements, future roof access requirements, lighting, glazing and electrical design, etc.

The structural framing components detailed in these drawings have been designed in accordance with the following documents for the design criteria detailed in these notes

Governing Building Code Loading Standards

2022 National Construction Code – Building Code of Australia Volume 2 and 2022 Housing Provisions Standard AS/NZS 1170.0:2002(+A5)

AS/NZS 1170.1:2002(+A2) AS/NZS 1170.2:2021

Cold formed Steel member standard AS/NZS 4600:2018

- 2.2 These drawings are also the limit of the Structural Design, any requirements for additional structural design of other items included in the project are specifically excluded if not shown on these drawings. This includes, but not limited to, requirements for additional loads that aren't specified including flood design loads, additional roof loads from solar panels, retaining walls required on site, driveway design etc.
- 2.3 These structural drawings and specifications represent the finished structure. The building is not considered complete until the installation of all components and details shown herein are installed according to the drawings.
- 2.4 No alterations are to be made to this structure without written approval of the engineer. This includes, but not limited to, modification to the plans and/or specifications, be the installation of additional openings, increased roof loads, skylight roof sheets or removal of cladding. If changes are made without written approval, such changes shall the legal and financial responsibility of the contractor or sub-contractors involved and it shall be their full responsibility to replace or repair the condition of the building as directed by the engineer

#### 3.0 Design Criteria

Building class	10a
Building Importance level	. 2
Wind region	A4
Terrain category	2.5
Topographic multiplier	. 1.16
Shielding multiplier	0.94
Ultimate design wind speed	42.7 m/s
Snow load	6.81 kPa
Slab imposed load	2.5 kPa or 9kN applied over 0.3x0.3m area (light vehicles)
Allowable bearing capacity of foundation supporting footings	100 kPa
Allowable bearing capacity of foundation supporting slab	50 kPa
Allowable skin friction of foundation	. 25 kPa
Soil Type	Non-aggressive (not saline or acid sulfate)

### 4.0 Installation Building Contractor Responsibilities

- 4.1 The contractor shall verify and confirm all site conditions and dimensions. Any discrepancies between drawings and site conditions shall be referred to the engineer for decision before proceeding with the work.
- 4.2 All workmanship and materials are to be in accordance with the Governing Building Code including all relevant Australian Standards and local statutory authorities except where varied by the contract documents.

4.3 The contractor shall be responsible for maintaining the structure in a stable condition and ensuring no part is overstressed under construction activities They shall provide all temporary bracing, shoring or other means to avoid excessive stresses and to hold structural elements in place during erection. These temporary provisions shall remain in place until sufficient permanent members are erected to ensure the safety of partially erected structures. The contractor is responsible for meeting all laws regulating the erection of steel buildings including, but not limited to, Safe Work Australia guidelines.

4.4 The contractor shall be responsible for the location of all services in the vicinity of the works. Any services shown are provided for information only.

The contractor shall confirm the location of all services prior to commencing and shall be responsible for the repair of any damage caused to services, as well as any loss incurred because of the damage to any service.

### 5.0 Foundation

- The bearing capacity of the foundation supporting the footings and slab shall be confirmed before any concrete is placed.
- 5.2 No earth or debris is to fall into the footings or piers before and during placing of concrete.
- 5.3 All footings shall be located centrally under walls and columns unless noted otherwise.
- 5.4 Concrete embedment depths do not apply to locations where any uncompacted fill or disturbed ground exists or where walls of the excavation
- will not stand without support. Request further advice from the engineer in these circumstances.
- 5.5 Fill used for the support of a slab on ground shall be controlled fill or rolled fill as in accordance with clause 6.4.2 of AS 2870-2011.
- 5.6 Slabs less than 100sq.m in plan area are suitable for AS 2870-2011 site classes A, S & M. For larger slabs or for site classes M-D, H1, H1-D, H2, H2-D, E & E-D, the slab may experience cracking more than is considered normally acceptable. The cracking is considered of aesthetic concern only and should not effect the structural performance of the slab or shed. If this is not desired, contact the engineer for further advice.

#### 6.0 Concrete

- Concrete placement and workmanship shall be in accordance with AS 3600-2018 & AS 2870-2011.
- 6.2 Concrete shall be
- a) N25 with slump of 100 mm in accordance with AS 1379-2007, with 20 mm maximum nominal aggregate size and no admixtures.
- b) consolidated by mechanical vibration
- c) Cured for a minimum of 7 days using continuous ponding with potable water.
- 6.3 No holes, chases or embedment of pipes other than those shown on the drawings shall be made in concrete members without prior approval of the engineer.

#### 7.0 Reinforcement

- Reinforcement shall comply with AS/NZ 4671-2019.
- 7.2 Reinforcement is represented diagrammatically and not necessarily shown in true projection.
- 7.3 Welding of reinforcement shall not be permitted without the approval of the engineer
- 7.4 All reinforcement shall be securely supported in its correct position ensuring the correct cover during placing of concrete by approved bar chairs, spacers or support bars. Approved chairs include stainless steel or plastic bar chairs for bottom reinforcement and plastic tipped wire bar chairs for top reinforcemer All chairs to be spaced at maximum of 750mm centres.

#### 7.5 Cover to reinforcment shall be:

- a) 50mm for surfaces of concrete in contact with the ground;
- b) 30mm for top surfaces of slabs fully enclosed by the building without open bays or
- c) 60mm for top surfaces of slabs more than 1 km from the coastline with open bays.
- d) For buildings with open bays within 1km of the coast, contact the engineer for cover and concrete grade requirements
- 7.6 Reinforcement shall be lapped 500mm for 12mmØ bars and 800mm for 16mmØ bars.
- 7.7 Mesh reinforcement shall be lapped such that the two outermost wires of one sheet overlap the two outermost wires of the other sheet by 25 mm.
- 7.8 Hooks, bends and cogs to be in accordance with AS 3600-2018 unless noted otherwise on drawings.

#### 8.0 Anchor Bolts

- 8.1 All anchors bolts shall be installed in accordance with the manufacturer's installation instructions.
- 8.2 Drill holes using a percussion drill (coring not permitted) to the correct hole diameter and depth as specified in the drawings.8.3 Thoroughly clean and blow the dust out of the holes using the cleaning accessories prescribed by the manufacturer's instructions.
- 8.4 Substitution of anchors bolts and chemical epoxy adhesive is not permitted unless written confirmation from the engineer is provided
- 8.5 For chemical anchors, ensure load is not applied to the anchors whilst epoxy adhesive is curing.

#### 9.0 Light Gauge Cold-formed Steel 9.1 All light gauge cold-formed steel shall comply with AS 1397-2021 and be the following grades

Thickness(mm)	Steel grade (yield stress, MPa	) Protective coating (g/m2)
BMT ≤ 1.0mm <sup>′</sup>	G550	Z350
1.0mm < BMT < 1.5m	m G500	Z350
1.5mm ≤ BMT ≤ 3.0m	m G450	Z350

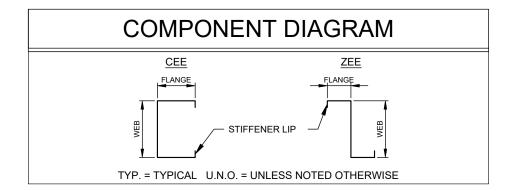
- 9.2 Welding of light gauge cold-formed steel shall not be permitted. 9.3 Column and rafter members shall not be drilled or notched without prior approval of the engineer
- 9.4 Round holes may be drilled through any girt or purlin member within the middle third of the depth of that member and not within 600mm of member end unless noted otherwise.
- 9.5 All bolts used to connect light gauge cold-formed steel members shall be
- a) Zinc coated M12 (min.) grade 4.6 snug tightened complying to AS 1111.1-2015 & AS 1112.3-2015 unless noted otherwise.
- b) Spaced no less than 3 bolt diameters between centres.
- c) Located no less than 1.5 bolt diameters from bolt centre to the end or edge of any light gauge member
- 9.6 All screws used to connect light gauge cold formed steel members (excluding sheeting) shall be
- a) 10g (min.) self-drilling screws complying with AS 3566.1-2002.
- b) Corrosion resistance class 4 in accordance with AS 3566.2-2002 for buildings within 1 km from the coastline with open bays or class 3 otherwise.
- c) Spaced no less than 3 bolt diameters between centres.
- d) Located no less than 1.5 bolt diameters from bolt centre to the end or edge of any light gauge member

### 10.0 Roof & Wall Sheeting

- 10.1 Roof & wall sheeting shall comply with AS 1397-2018 and have suitable corrosion protection complying with Table 7.2.2a of the 2022 Housing Provisions Standard.
- 10.2 During construction and maintenance, no foot traffic shall occur within end spans of sheeting, foot traffic shall occur
- a) Evenly across at least two ribs for corrugated profiled sheeting or
- b) In the pans for pan-type profiled sheeting.
- 10.3 Any roof skylights shall be approved by the engineer
- 10.4 Safety mesh shall be installed in accordance with the building code

### 11.0 Door & Window Components

- 11.1 Wind-locked roller doors are assumed to remain in-place and resist the ultimate limit state wind loading except for in cyclonic regions
- 11.2 Non-wind-locked roller doors are assumed to have failed at the ultimate limit state wind loading
- 11.3 Personal access doors shall be rated for the wind loading parameters stated in the design criteria (see section 3.0)
  11.4 All windows shall be in accordance with AS 1288-2021 & AS 2047-2014(+A2) as appropriate for the wind loading parameters stated in the design criteria (see section 3.0)



REV	DATE	DESCRIPTION
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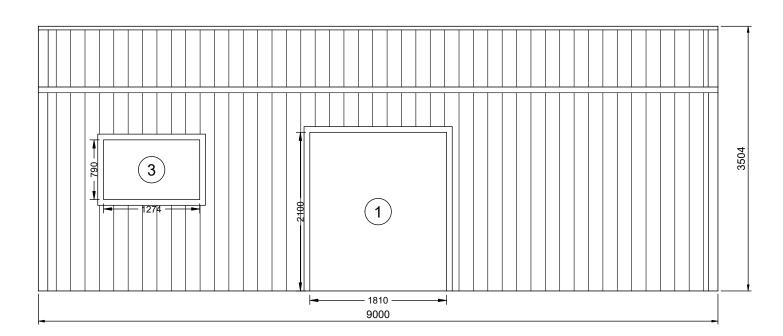
PO Box 3084 THIRROUL NSW 2515 Signed sheds@venn.engineering ABN 39 626 802 257



Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena

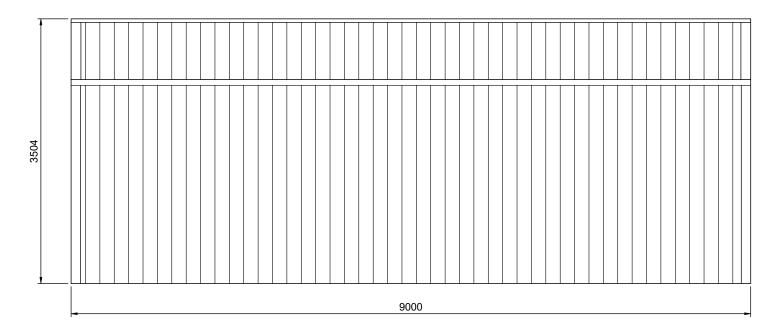
TAS, 7030

24-04-2025 JOB NO. HGOR96973477 SHEET 1 of 11



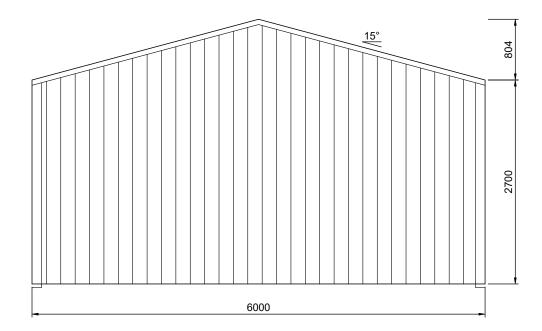
# SIDEWALL B BUILDING ELEVATION

SCALE: 1:50



SCALE: 1:50



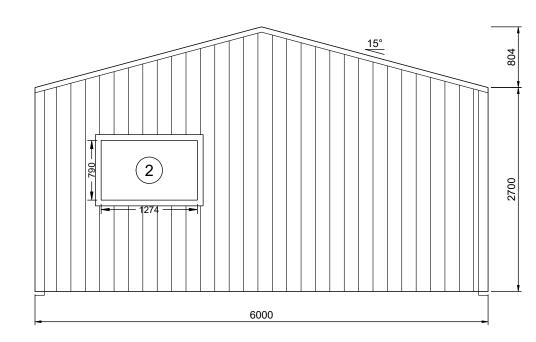


# **REAR BUILDING ELEVATION**

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FRAME #4





# FRONT BUILDING ELEVATION

SCALE: 1:50

FRAME #1

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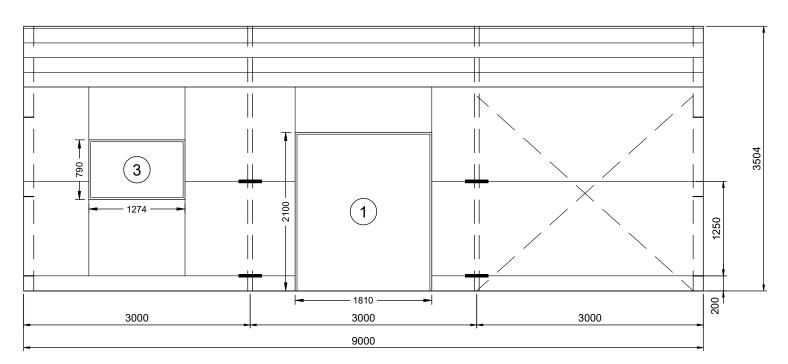
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PO Box 3084
THIRROUL NSW 2515
Grant LWood

Grant LWood Grant J Wood MIEAust CPEng NER RPEQ

Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030

24-04-2025 JOB NO. HGOR96973477 SHEET 2 of 11

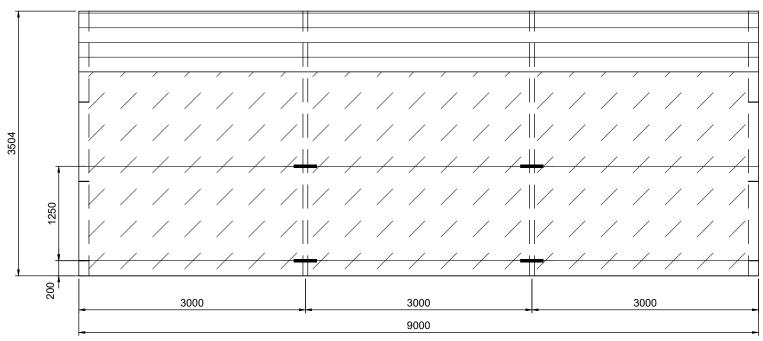


# SIDEWALL B FRAMING ELEVATION

SCALE: 1:50

# DIAPHRAGM SCHEDULE SHEETING IN DIAPHRAGM SECTIONS (SHOWN AS HATCHED AREA ON ELEVATIONS) NOT TO BE CUT UNDER ANY CIRCUMSTANCES

WALL	DISTANCE FROM WALL EDGE
Sidewall 'A'	0-9000





# SIDEWALL A FRAMING ELEVATION

sheds@venn.engineering ABN 39 626 802 257

SCALE: 1:50

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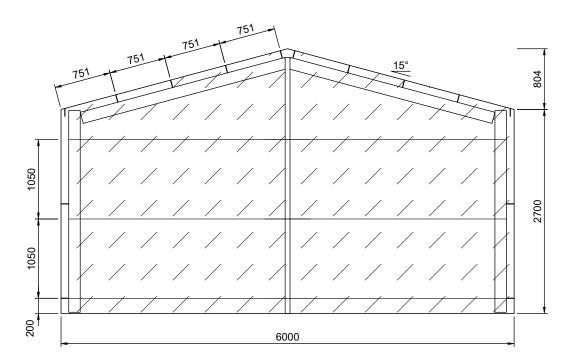




Grant J Wood MIEAust CPEng NER RPEQ Registered EA Chartered Professional Enginer (No. 2383009)
Registered Professional Enginer QLD (No. 14384)
Registered Professional Enginere QLD (No. 14384)
Registered Child Enginere Building Practitioner VIC (No. PE0002499
Registered Child Enginere (structural) NT (No. 3063716)
Building Services Provider (Engineer Civil) TAS (No. 6509369425)

Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030

DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 3 of 11



# REAR FRAMING ELEVATION

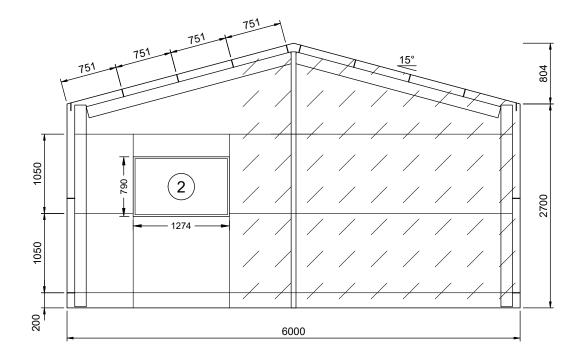
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FRAME #4

### DIAPHRAGM SCHEDULE

SHEETING IN DIAPHRAGM SECTIONS (SHOWN AS HATCHED AREA ON ELEVATIONS) NOT TO BE CUT UNDER ANY CIRCUMSTANCES

WALL	DISTANCE FROM WALL EDGE
Endwall 'A'	0-3850
Endwall 'B'	0-6000



sheds@venn.engineering ABN 39 626 802 257

# FRONT FRAMING ELEVATION

SCALE: 1:50

FRAME #1

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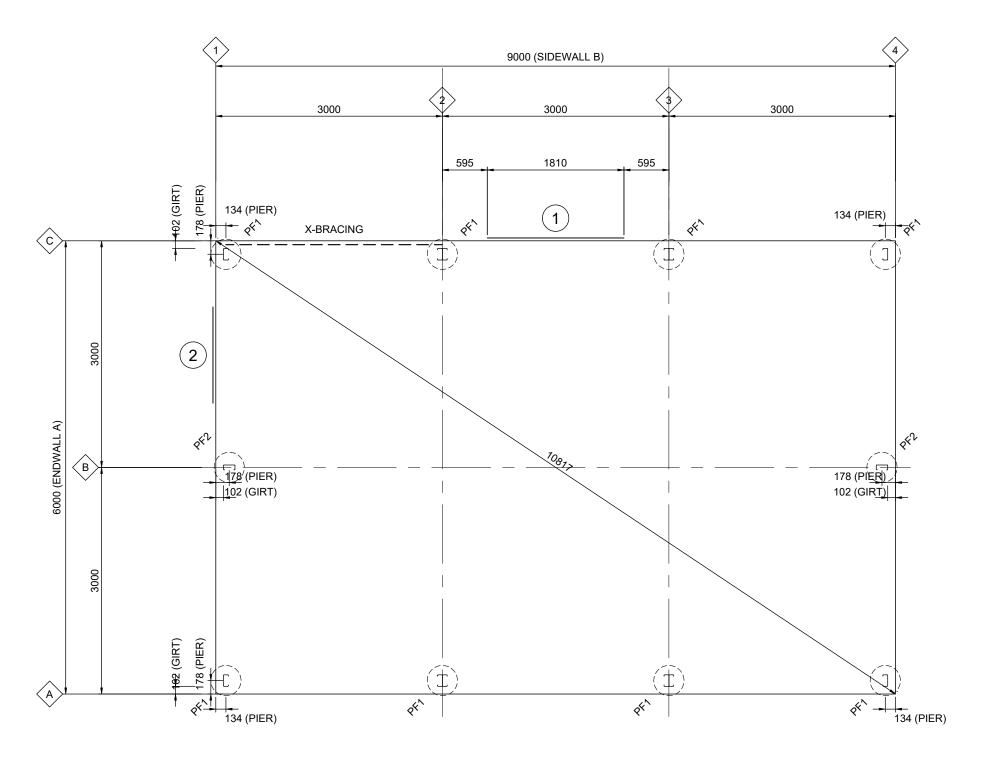




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THIRROUL NSW 2515
Grant L Wood

Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030

DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 4 of 11



# 1 FOOTING/SLAB FLOOR PLAN

SCALE: 1:50

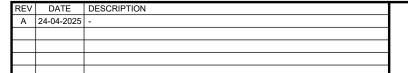
PF1 - 400Ø REINFORCED CONCRETE PIERS TO DETAIL PF2 - 400Ø REINFORCED CONCRETE PIERS TO DETAIL

SLAB IS DESIGNED FOR CARS AND LIGHT VANS NOT EXCEEDING 3500kg GROSS MASS

CONCRETE CONTROL JOINTS SHALL BE PROVIDED IN SLAB TO DETAIL AT NOT MORE THAN 10m CENTRES IN EACH DIRECTION, APPROXIMATELY EQUALLY SPACED AND LOCATED APPROXIMATELY MIDWAY BETWEEN COLUMNS/MULLIONS

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Customer Name: Paul Chambers

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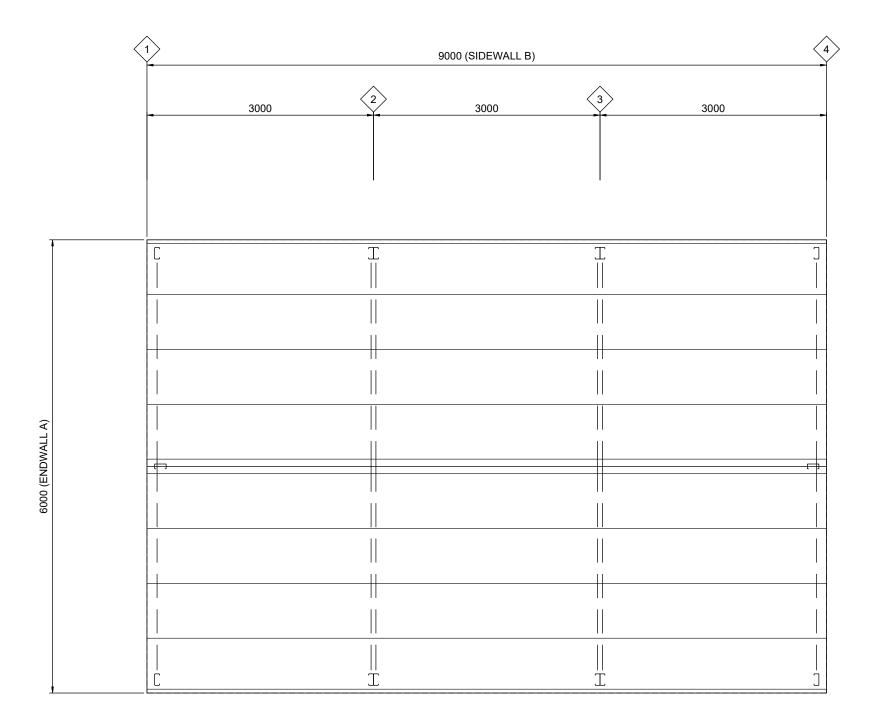
Customer Name: Paul Chambers

45 Cider Gum Road

Miena,

TAS, 7030

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# **ROOF FRAMING PLAN**

SCALE: 1:50

ROOF SHEETING IS USED AS DIAPHRAGM TO BRACE THE BUILDING AND IS NOT TO BE CUT UNDER ANY CIRCUMSTANCES

sheds@venn.engineering ABN 39 626 802 257

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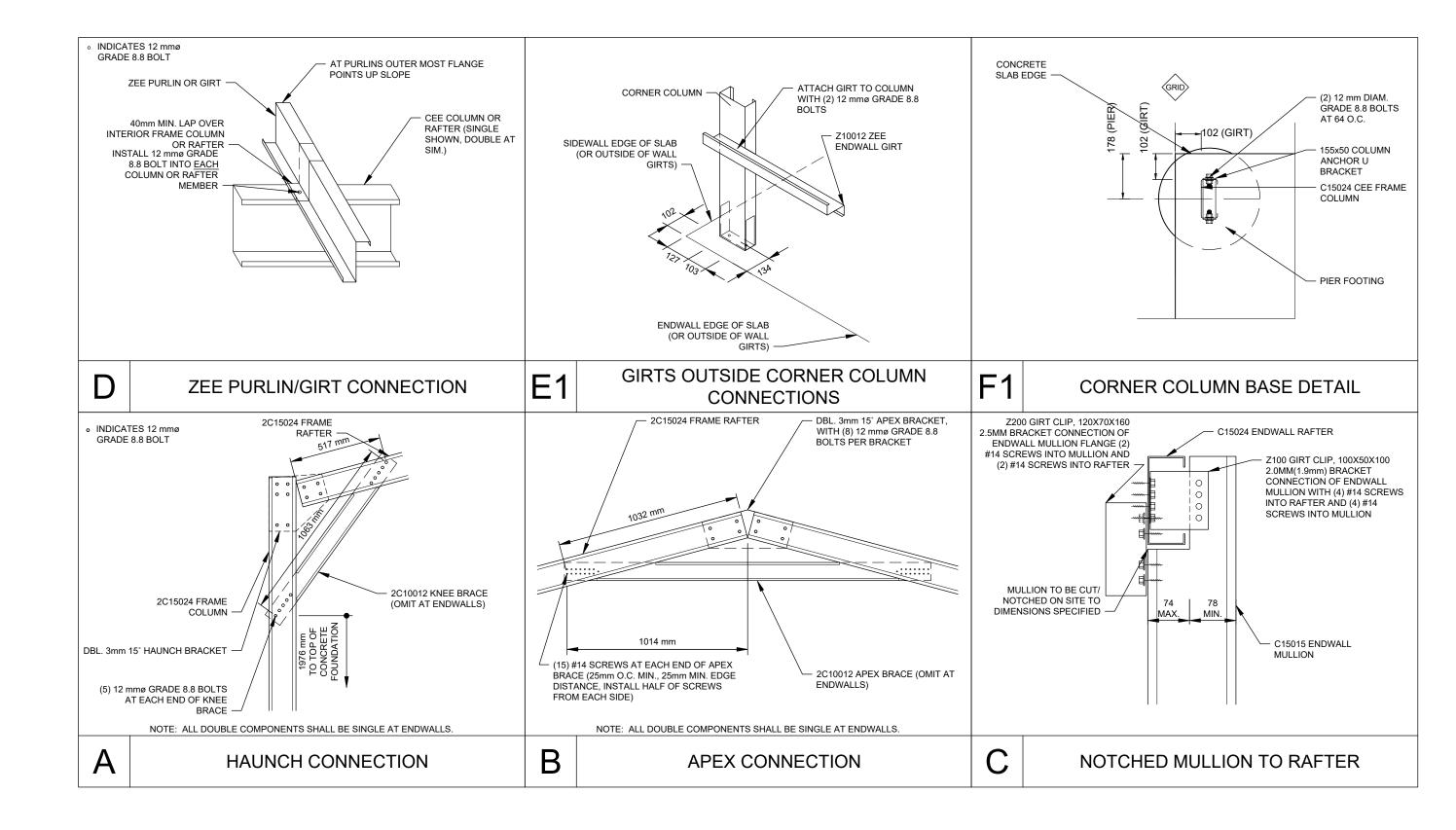


PO Box 3084 Signed Date 24-04-2025
Sheds@vienn and a signed Grant I Wood Grant J Wood MIEAust CPEng NER RPEQ
Registered EA Chartered Professional Engineer (No. 2383009)
Registered Professional Engineer (No. 14384)
Registered Chile Engineer Building Practitioner (Vic (No. PE0002499)
Registered Certifying Engineer (structural) NT (No. 306371ES)
Building Services Provider (Engineer Civil) TRAS (No. 806950425)

Miena, TAS, 7030

Customer Name: Paul Chambers Site Address: 45 Cider Gum Road

DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 6 of 11



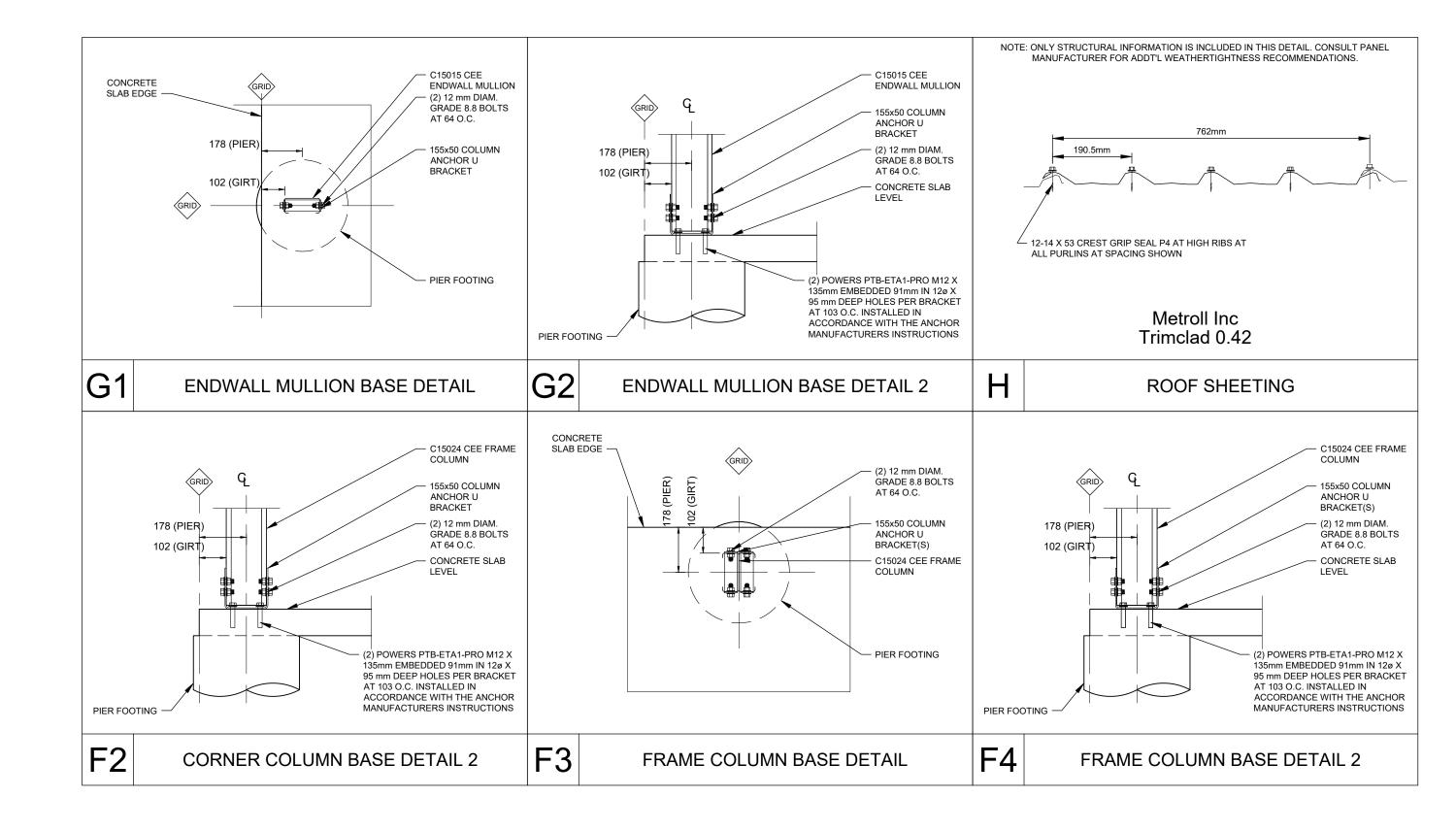
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Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030 DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 7 of 11



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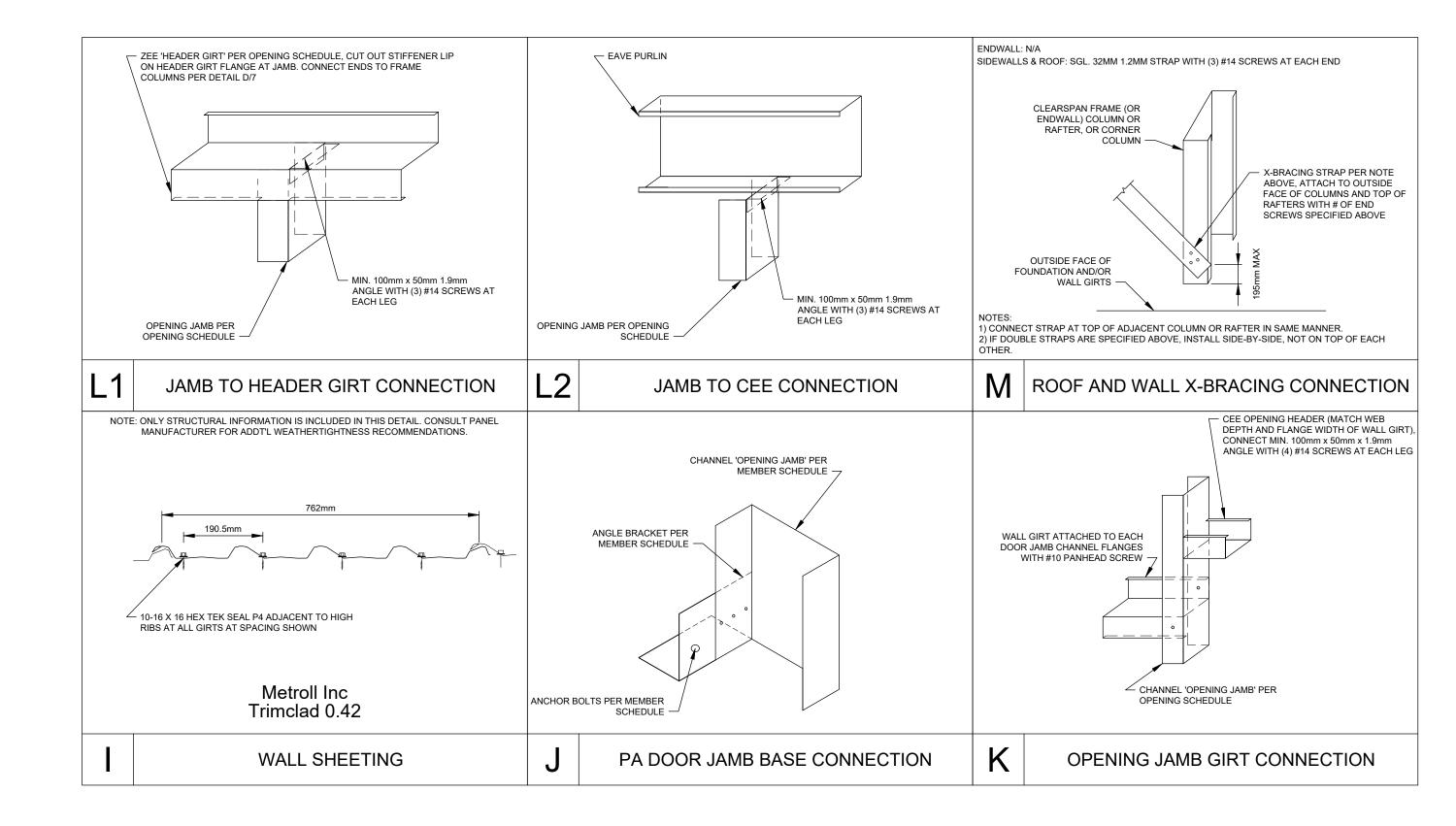






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Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030 DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 8 of 11



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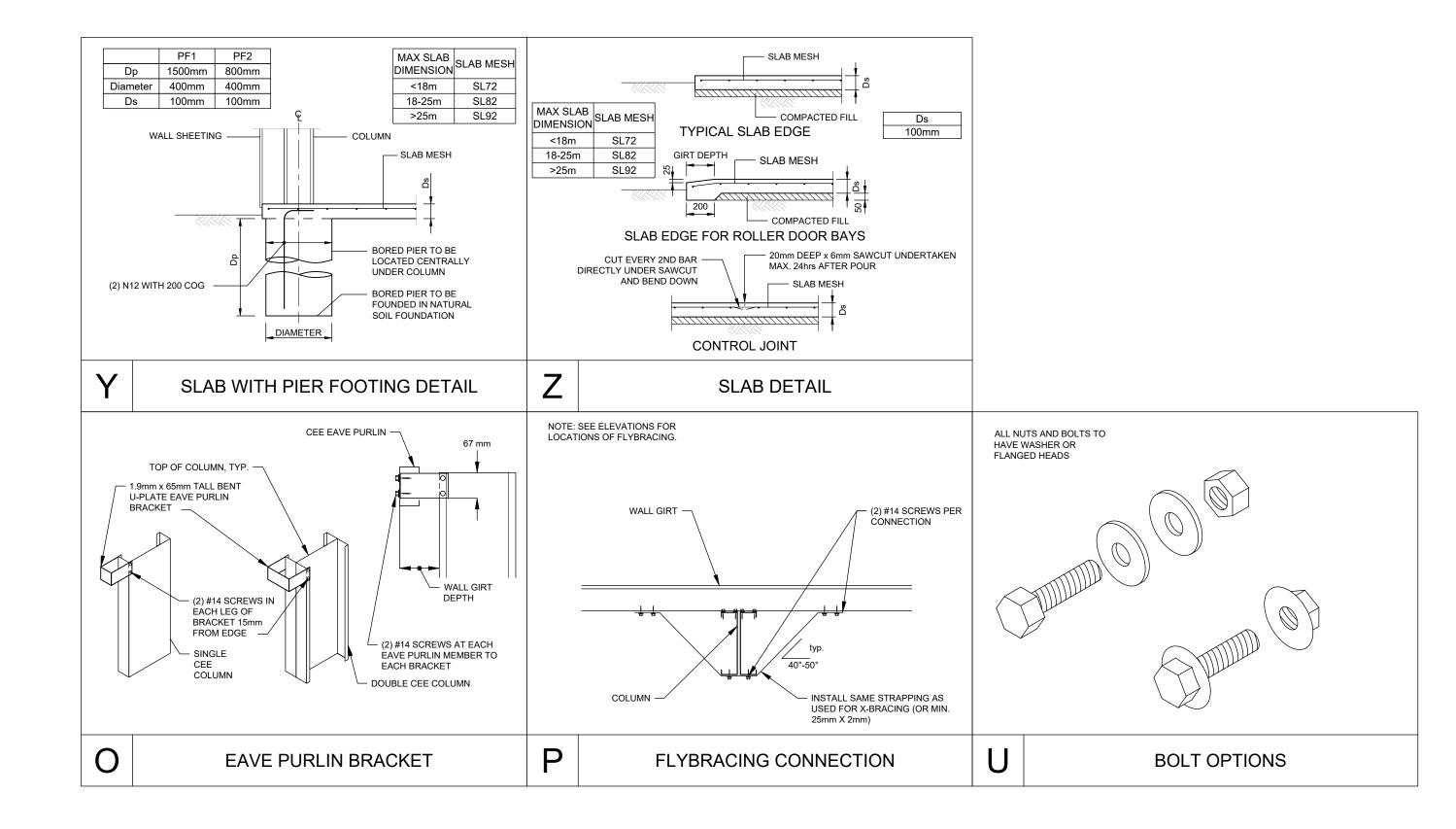


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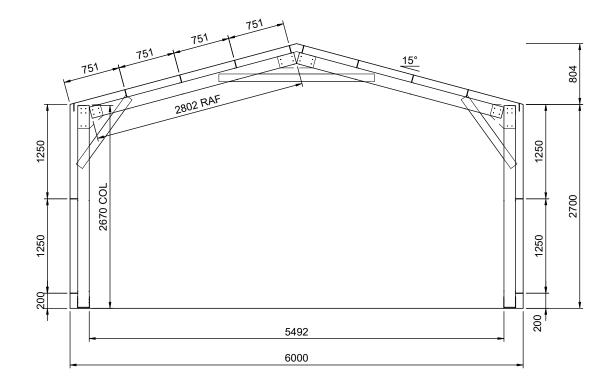






Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030

DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 10 of 11



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# **INTERNAL FRAMING ELEVATION**

SCALE: 1:50 FRAMES 2, 3

REV	DATE	DESCRIPTION
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Date 24-04-2025

Grant J Wood MIEAUS (PEND NER RPEQ

STANDARD STAN

**ROOF PURLINS** 

**EAVE PURLIN** 

SIDEWALL GIRTS

ENDWALL GIRTS

COMPONENT

MEMBER

BASE CONNECTION

MEMBER

BASE CONNECTION

MEMBER

BASE CONNECTION

**MEMBER** 

BASE

CONNECTION

MEMBER

CLEAR SPAN PORTAL

(FRAMES 2, 3)

**ENDWALL PORTAL** 

(FRAMES 1, 4)

ENDWALL MULLION

OPENINGS (1-2)

OPENING (3)

X-BRACING

Customer Name: Paul Chambers Site Address: 45 Cider Gum Road Miena, TAS, 7030

MEMBER SCHEDULE

RAFTER

COLUMN

APEX BRACE

KNEE BRACE

BRACKET TYPE

ANCHOR BOLTS

RAFTER

COLUMN

APEX BRACE

KNEE BRACE BRACKET TYPE

ANCHOR BOLTS

COLUMN

**BRACKET TYPE** 

ANCHOR BOLTS

MEMBER

MEMBER

MEMBER

MEMBER

JAMB

HEADER/SILL

BRACKET TYPE

ANCHOR BOLTS

JAMB

HEADER/SILL

STRAP

DATE 24-04-2025 JOB NO. HGOR96973477 SHEET 11 of 11

TYPE

Double C15024

Double C15024

Double C10012

Double C10012

Base cleat bolt down bracket BC.150

(4) Powers PTB-ETA1-PRO M12 x 135mm embedded 91mm

Single C15024

Single C15024

Base cleat bolt down bracket BC.150

(2) Powers PTB-ETA1-PRO M12 x 135mm embedded 91mm

Single C15015

Base cleat bolt down bracket BC.150

(2) Powers PTB-ETA1-PRO M12 x 135mm embedded 91mm

Single Z10019 @ 751mm centres

Single C10019

Single Z10012 @ 1250mm centres

Single Z10012 @ 1050mm centres

Single Unlipped 102 x 1.5 Cee

Single C10012

Angle base connection ABC.SINGLE

(1) Dewalt Blue-tip screw bolt BT12 x 75mm embedded 70mm

Single Unlipped 102 x 1.5 Cee

Single C10012

32mm x 1.2 strap

		Signed .
	THIRROUL NSW 2515	Ū
	sheds@venn.engineering	l
ING /	ABN 39 626 802 257	

### Generic Temporary Bracing Information

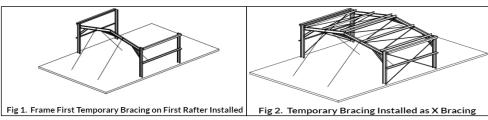
The installation of temporary bracing is critical to avoid building collapse or damaging structural movement during construction. This collapse can occur with no notice and as such the installation of appropriate temporary bracing is critical to avoid damage, injury, and possible death. Determination, procurement, and correct installation of temporary bracing is the responsibility of the builder / primary contractor / installar

### **Bracing Materials**

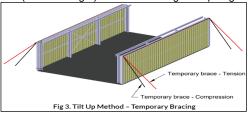
The constructor / installer is to supply suitably sized materials for temporary bracing. These materials are generally capable of tension, but in some circumstances will need to be capable of tension and compression. Load rated ratchet strapping of an appropriate size can be used to temporarily 'x-brace' bays in both directions, until the final bracing systems are fullyinstalled. This is especially critical for buildings where X Bracing is not required in the final structure due to the use of moment frames or diaphragm bracing.

### **Temporary Bracing Location**

The location of Temporary bracing will depend on the installation method used. Installation should be completed in accordance with the Construction Package, Engineering Plans, and Instruction Manuals. If the Frame First Method (most common) is used, then the use of tension only bracing and creating temporarily braced bays as per Fig 1 and Fig 2. can be used. As a basic guide, a minimum of every 4th bay should have temporary bracing installed as per Fig 2.



If the Tilt Up Method Is used (where walls are constructed on the ground And then tilted into place), then the tops of columns are braced with a tension and compression brace in the same direction Fig 3. Then rafters and purlins can be installed with temporary bracing holding rafters in place (similar to Fig 1) until final bracing of diaphragm sheeting is installed.



Typically, braces should be positioned diagonally across the structure from the top to the bottom, intersecting near the midpoint to provide stability, optimally at a 45-degree angle but no less than a 20-degree angle. The connection strength of temporary bracing is a critical consideration and these connections must be capable of resisting the potentially substantial temporary bracing loads – whether this connection point be to the building, the foundations or to the ground. Dependent upon building size this may include heavy angles and post installed concrete anchors. The temporary bracing methods used must be capable of fully stabilising the structure during the construction process.

### Additional Temporary Bracing

The temporary bracing described is a minimum requirement for a standard-sized building in average conditions. Additional consideration should be given to larger building spans and/or challenging site conditions. There may also be an increased risk in relation to partially completed buildings and exposed sites. It is recommended that extra temporary bracing is utilized if moderate wind speeds are expected on site. Additional support elements, such as steel cables may need to be introduced that can be attached to the building's framework and anchored to the ground or other stable structures to provide extra stability. The frame should remain rigid throughout and such responsibility lies with the constructor. Buildings should not be left in a partially completed state longer than necessary.

### **Bracing Removal**

The temporary bracing should not be removed until all purlins, girts and permanent cross bracing, diaphragm bracing or moment frames where used are installed. The temporary bracing is to remain in place where possible, until the roof and wall cladding is fully installed. If you need any further information regarding the installation of temporary bracing or are at all unsure of the necessary requirements for this specific building, there are guides available through various industry bodies:

https://www.safeworkaustralia.gov.au/ 'Construction work – steel erection. Information sheet', 2016. https://www.steel.org.au/ 'Structural steelwork fabrication and erection code of practice', 2014. https://www.standards.org.au/ AS/NZS 5131:2016 'Structural steelwork – Fabrication and erection.

Support is also available at support@actbuildingsystems.com.

THE ABOVE INFORMATION REGARDING TEMPORARY BRACING DOES NOT FORM PART OF THE ENGINEERING CERTIFICATION FOR THIS DESIGN AND IS PROVIDED AS A GUIDE TO AID INSTALLATION ONLY.